An engineering analysis of truss-out metal bracket bamboo scaffoldings

Dr. C.C. Chang
Department of Civil Engineering
Hong Kong University of Science and Technology

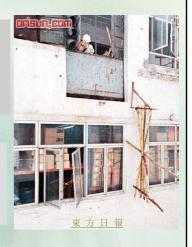
Use of truss-out bamboo scaffoldings

High accident and fatality rate

「狗臂架」 鬆 脫 肇 禍 判 頭 兄 長 悲 痛 搭 棚 工 失 足 墮 樓 喪 命 15 April 2005 蘋果日報

搭棚換窗 兩人跌死 14 July 2005 東方日報

兩名工人在工廠大廈五樓外牆搭建工作台,為廠房更換鋁窗的工程做準備,工作台疑地勝負荷坍塌,兩人無佩戴安全帶下,飛墮地面重傷,送院不治。勞工處職員在現場發現支撐工作台的「狗臂架」只繫上一粒爆炸螺絲



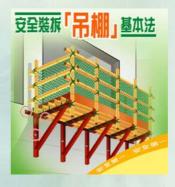
High accident and fatality rate

【本報訊】一顆鬆脫螺絲釀成一宗奪命意外!一名工人昨午在土瓜灣帝庭國,拆卸一個位於23樓的外牆竹棚時,懷疑承托竹棚其中一個狗狗門的一粒螺絲鬆脫,導致竹棚無法人重量倒塌,懸垂半空,工人竟適地地面重傷死亡,勞工處正調查意外原因。



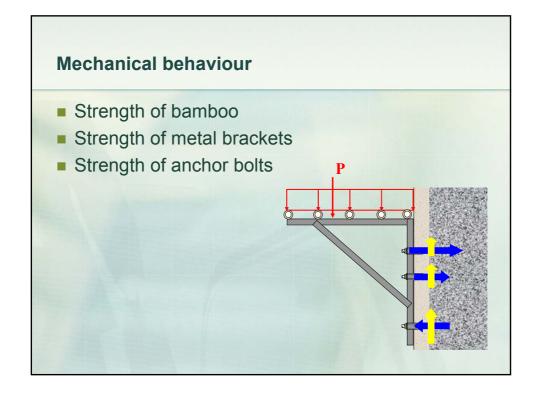
Problems of truss-out bamboo scaffoldings

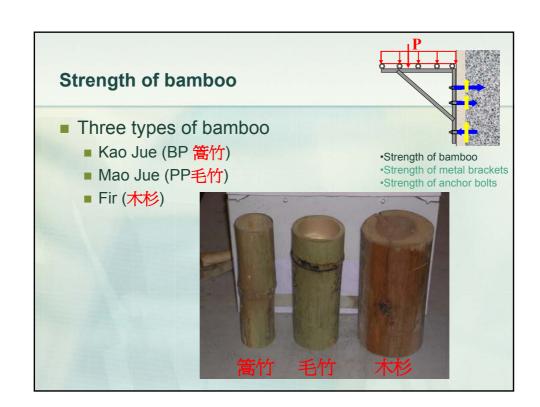
- Safe?
- Factors affecting their safety?
- Possible improvement?

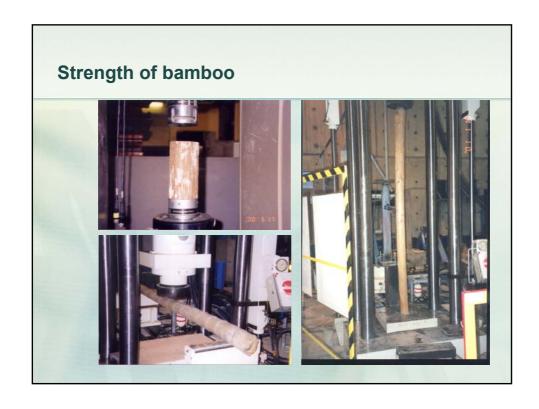


Load-carrying capacity of truss-out bamboo scaffoldings









Strength of bamboo

Mechanical properties for BP under normal compressive test

NL	D (r	nm)	A (n	nm²)		S (M	Pa)	E (C	GPa)
	mean	std	mean	Std	mean	std	95% prob.	mean	std
1/4	53.2	6.8	969.5	365.5	58.6	7.7	45.9	7.9	1.3
1/2	53.4	6.8	969.8	355.4	56.6	6.0	46.7	7.9	1.6
none	54.6	5.5	989.5	295.8	56.0	9.7	40.1	7.6	2.1

Concrete 40 MPa

25 GPa

Steel 500 MPa

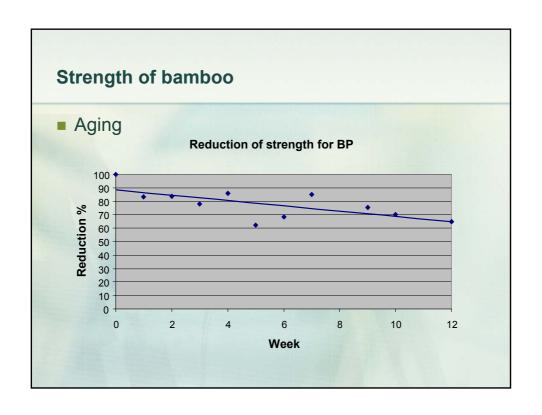
200 GPa

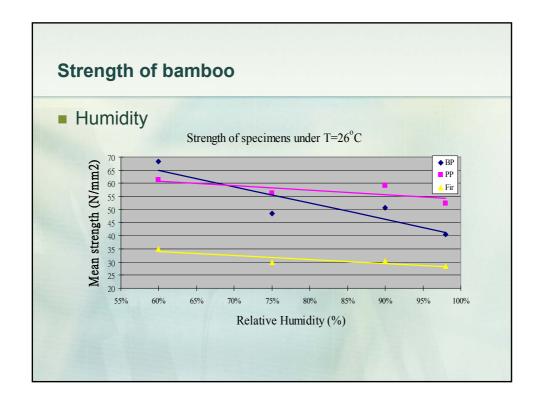
Strength of bamboo

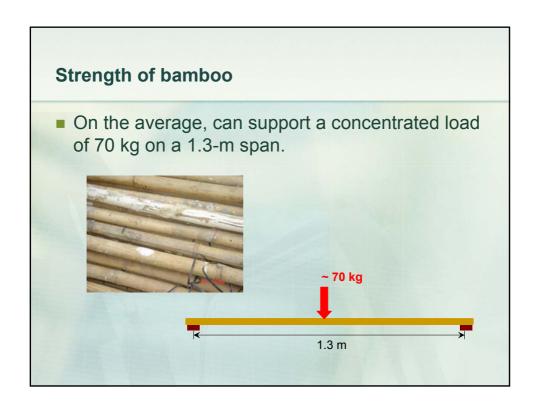
 Mechanical properties for BP under 3-point bending test

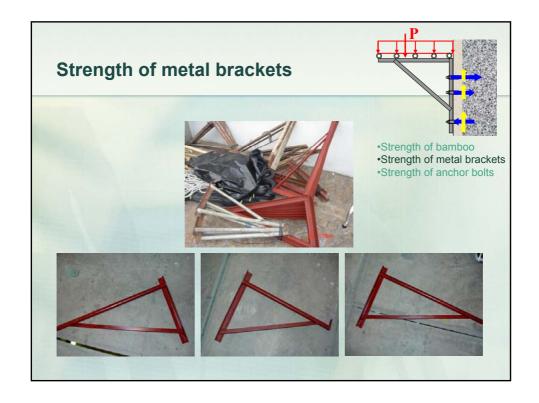
Material	E (GPa)	S (MPa)	
Fir	6.3	44.6	
PP	11.8	78.7	
BP	20.0	80.7	

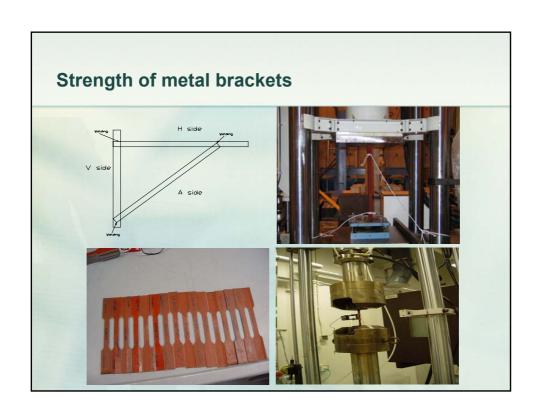
Concrete 25 GPa Steel 200 GPa 40 MPa 500 MPa

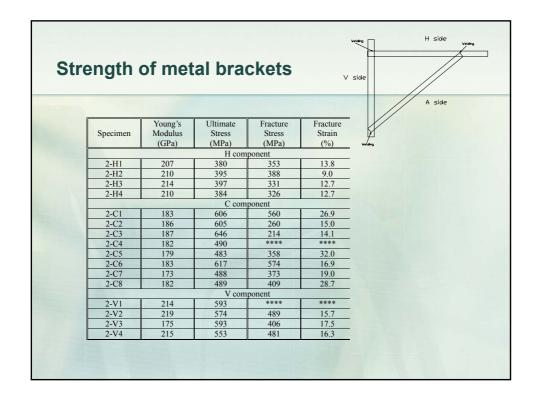


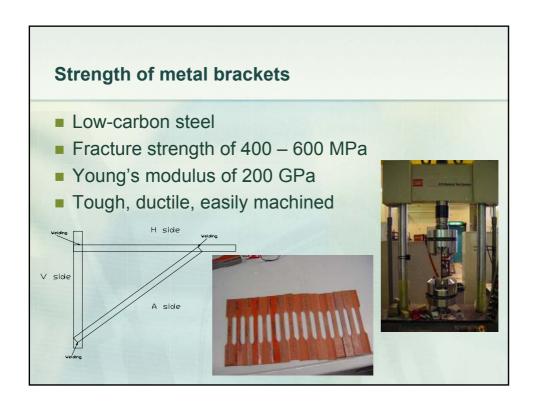


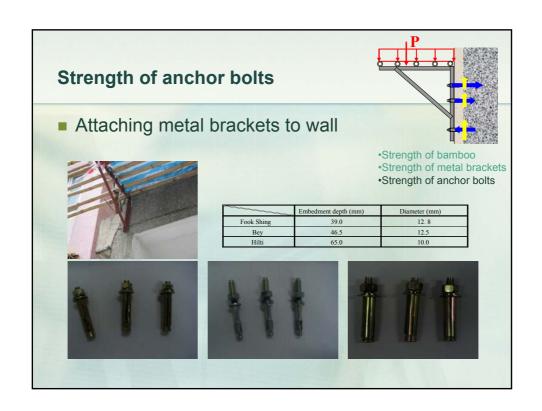


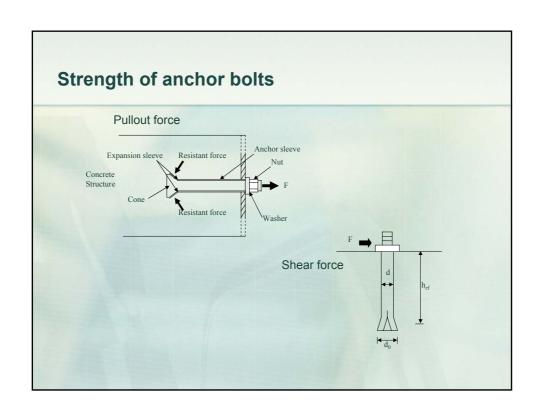


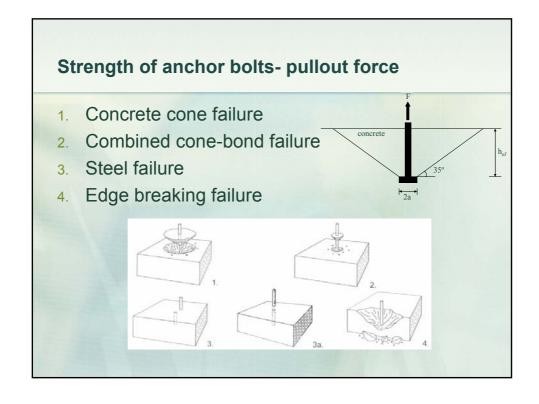


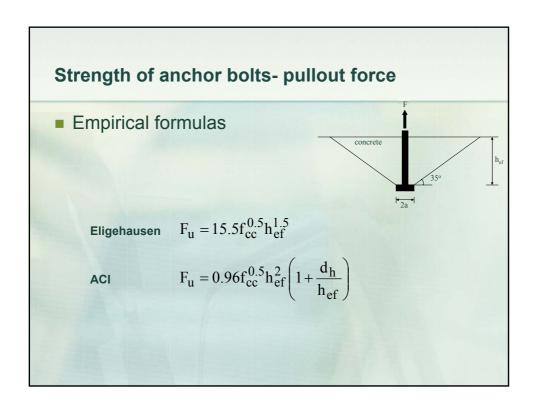


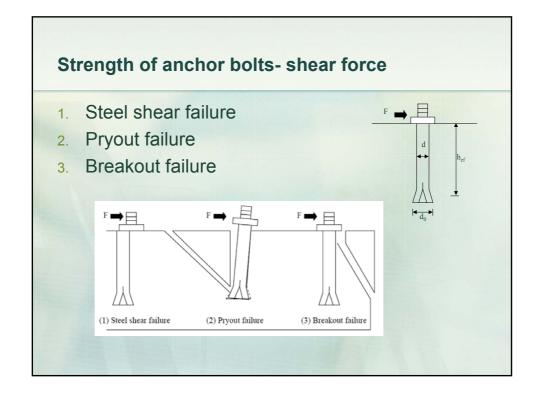












Strength of anchor bolts- shear force

Empirical formulas

Steel shear failure (剪力破壞)

$$V_s = 0.6 f_{ut} A_e$$

Pryout failure (撬離破壞)

$$V_{cp} = k_{cp} \psi_{Ng} \psi_{Ne} \psi_{Ncr} (12.5 \sqrt{f'_c} h_{ef}^{1.5})$$
 $h_{ef} < 280 \text{mm}$

$$h_{\rm ef} < 280 \, \rm mm$$

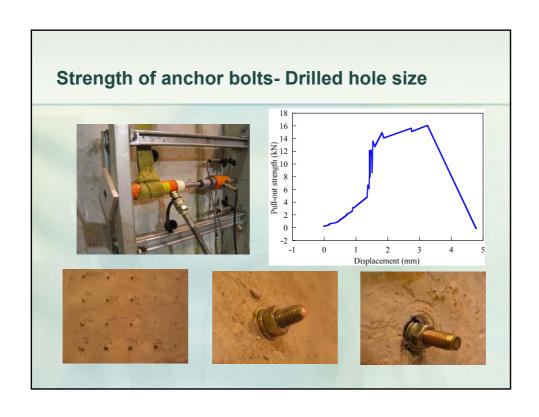
$$V_{cp} = k_{cp} \psi_{Ng} \psi_{Ne} \psi_{Ncr} (4.75 \sqrt{f_c'} h_{ef}^{1.67})$$
 $280 \le h_{ef} \le 635 \text{mm}$

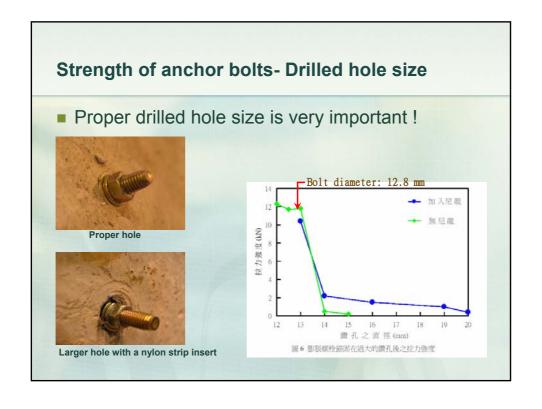
$$280 \le h_{ef} \le 635 \text{mm}$$

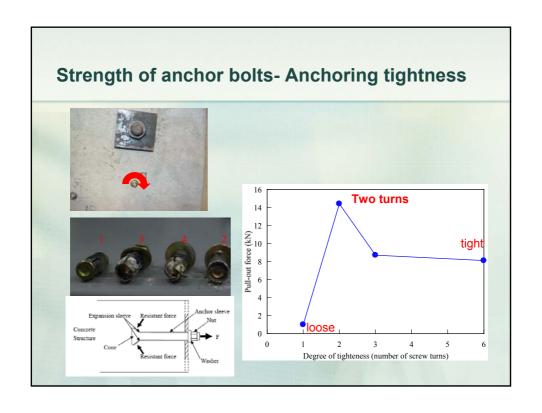
Breakout failure (斷裂破壞)

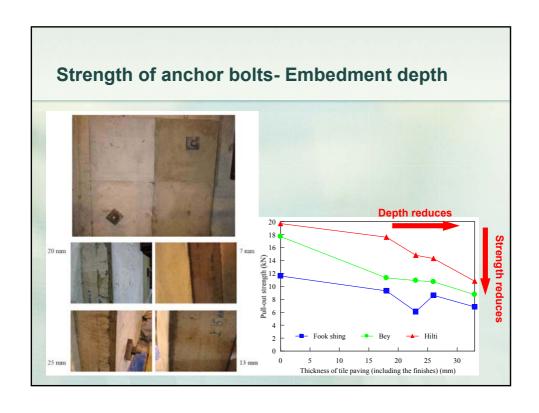
failure (斷裂破壞)
$$V_{n,c} = \psi_{Vg} \psi_{Ve} \psi_{Vcr} 0.84 \left(\frac{h_{ef}}{d}\right)^{0.2} \sqrt{d} \sqrt{f_c'} c_1^{1.5}$$

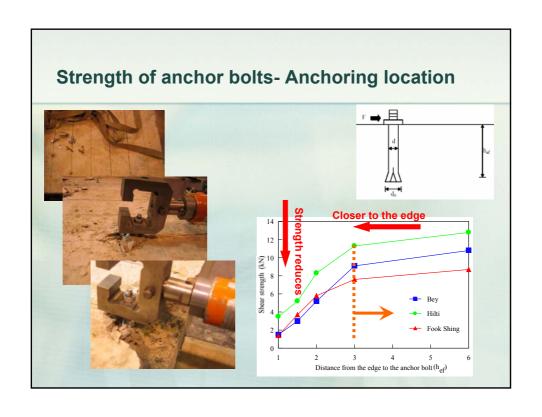
Strength of anchor bolts Pullout force Factors Drilled hole size Resistant force Expansion sleeve Anchoring tightness ■ Embedment depth Cone Resistant force Anchoring location Wall material Shear force

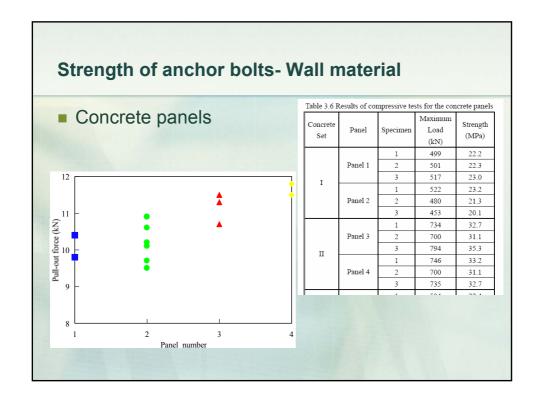


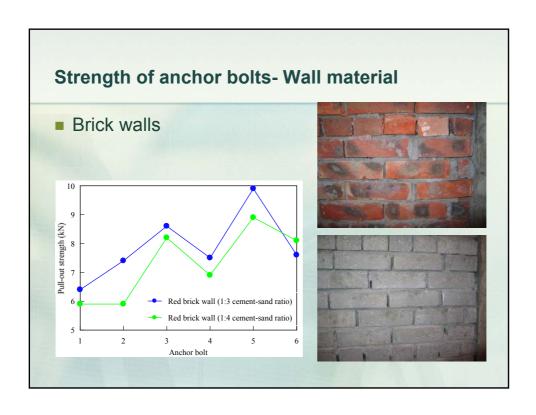


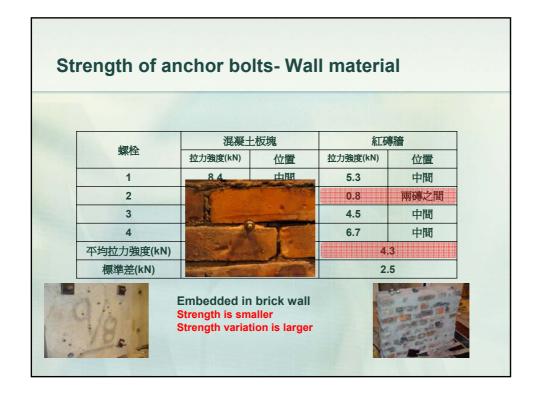


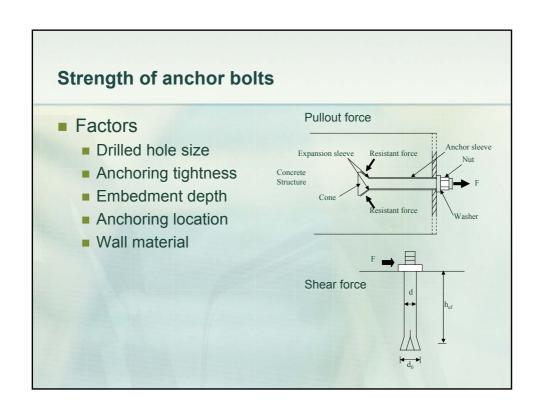


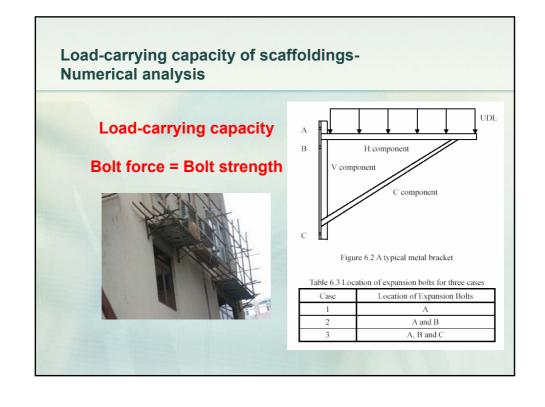












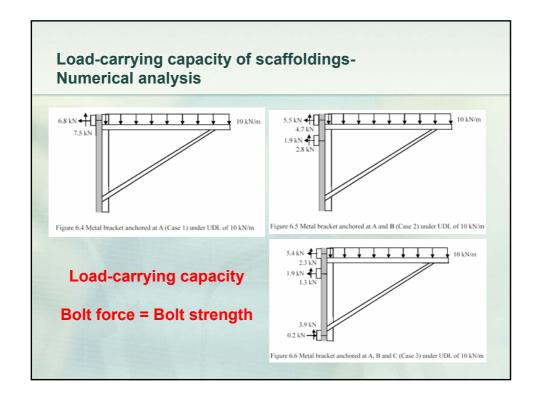
Load-carrying capacity of scaffoldings-Numerical analysis

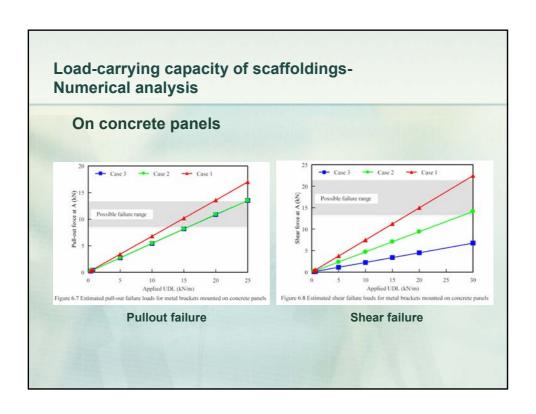
Table 6.1 Pull-out strength of the Fook Shing expansion bolts (in kN)

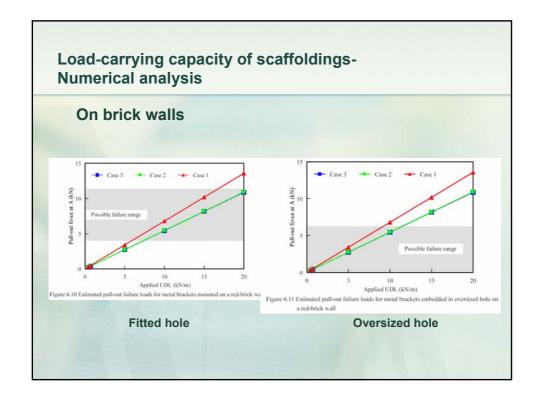
Base material	Hole	Range	Mean	Std	Mean-3Std	Mean+3Std
Concrete	Fitted	9.5 - 11.8	10.6	0.7	8.5	12.7
Concrete	Over-sized	0.4 - 2.2	1.3	0.8	0	3.7
Red-brick	Fitted	5.9 - 9.9	7.6	1.2	4.0	11.2
wall	Over-sized	0.5 - 3.4	1.7	1.3	0	5.6
Sand-brick	Fitted	6 - 9.5	7.4	1.9	1.7	13.1
wall	Over-sized	-	-	-		-

Table 6.2 Shear strength of the Fook Shing expansion bolts (in kN)

Base material	Hole	Range	Mean	Std	Mean-3Std	Mean+3Std
Concrete	Fitted	16.0 - 18.9	17.5	1.3	13.6	21.4
Concrete	Over-sized	-	-	-	-	-







Load-carrying capacity of scaffoldingsExperimental study



Load-carrying capacity of scaffoldings-Experimental study



Load-carrying capacity of scaffoldings-Experimental study

Anchored on concrete



Failure	Case 1	Case 2	Case 3
Failure UDL (kN/m)	7.1	15.6	20.1
Failure load (kN)	5.2	11.4	14.7

Lower when anchored on brick walls

Anchored on brick



Failure	Case 1	Case 2	Case 3
Failure UDL (kN/m)	5.8	10.1	12.2
Failure load (kN)	4.2	7.4	8.9

Load-carrying capacity of scaffoldings-Experimental study

Anchored in oversized holes (brick wall)

Hole	Failure	Case 1	Case 2	Case 3
Fig. 4	Failure UDL (kN/m)	5.8	10.1	12.2
Fitted	Failure load (kN)	4.2	7.4	8.9
Ossansia a d	Failure UDL (kN/m)	2.6	4.3	4.5
Oversized	Failure load (kN)	1.7	2.9	3.0



Oversized



Fitted

Load-carrying capacity of scaffoldings-Experimental study

Anchored on the edge/side

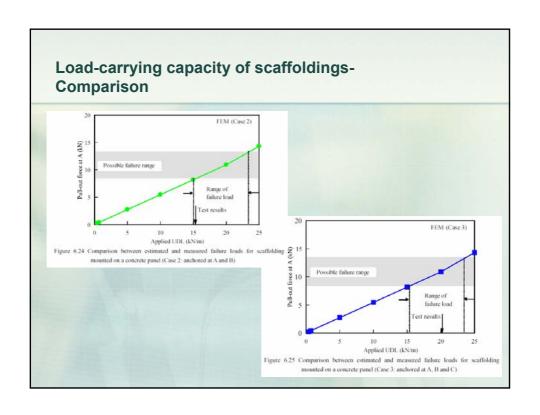


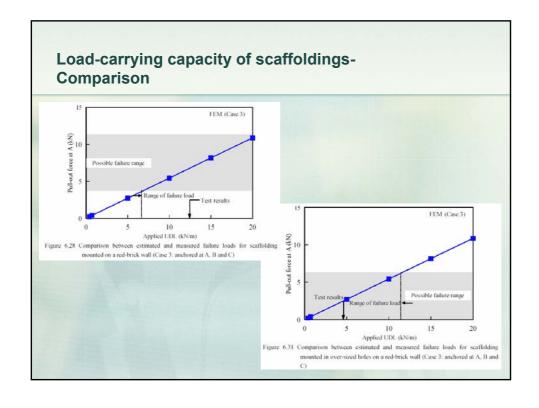
Anchored on the side

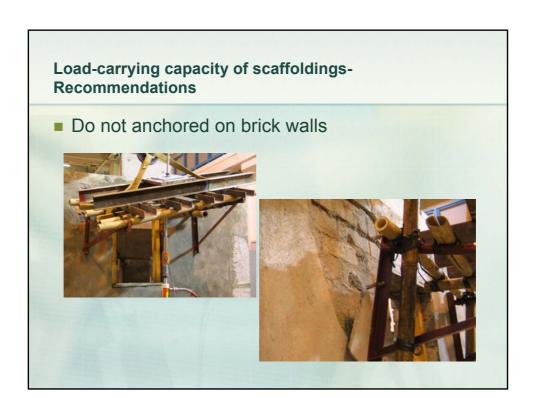
	Panel th	nickness
	100 mm	160 mm
Failure UDL (kN/m)	0.4	1.4
Failure load (kN)	0.3	1.0

Anchored on the wall

Failure	Case 1	Case 2	Case 3
Failure UDL (kN/m)	7.1	15.6	20.1
Failure load (kN)	5.2	11.4	14.7





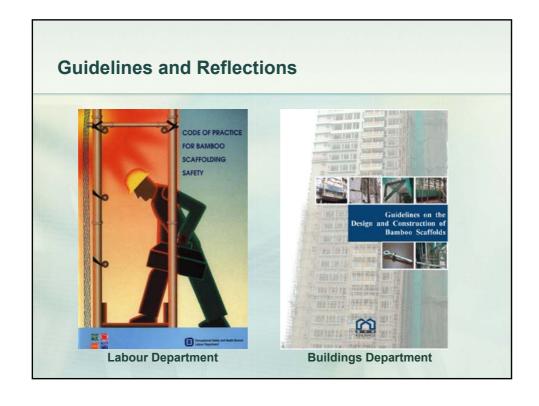


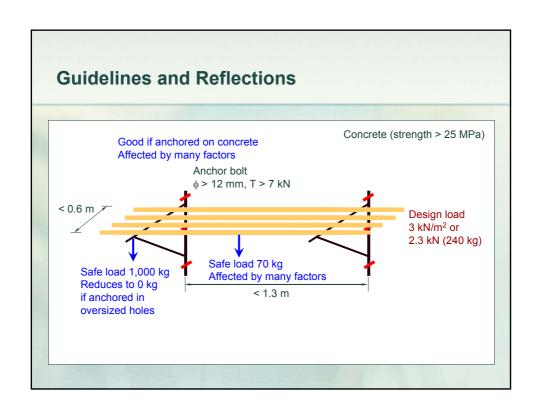




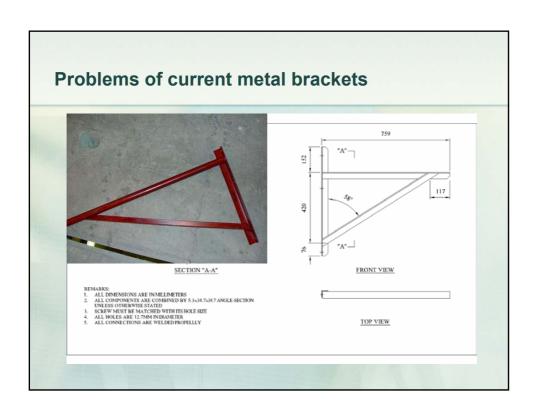


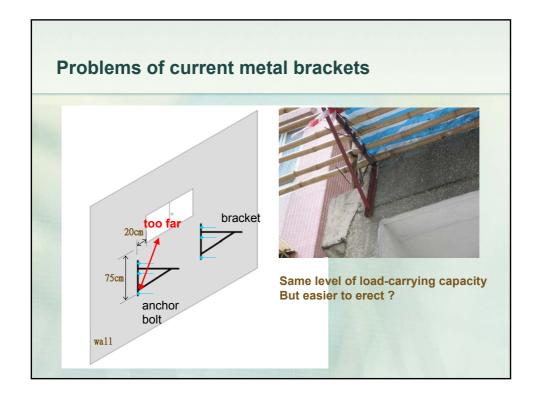
Load-carrying capacity of scaffoldings-Recommendations Anchored with proper tightness

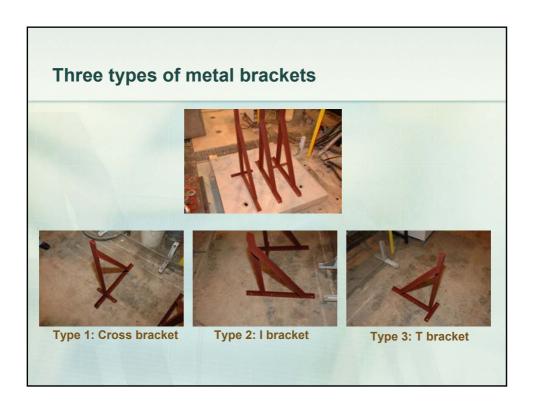




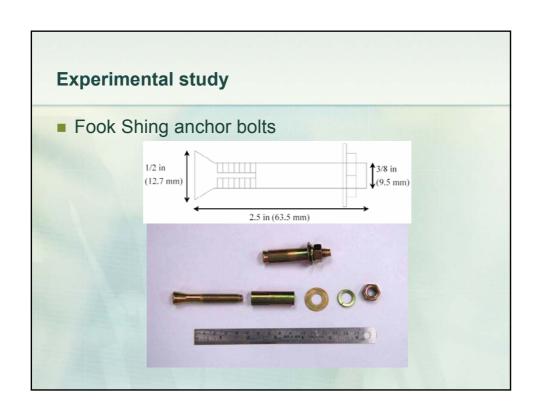


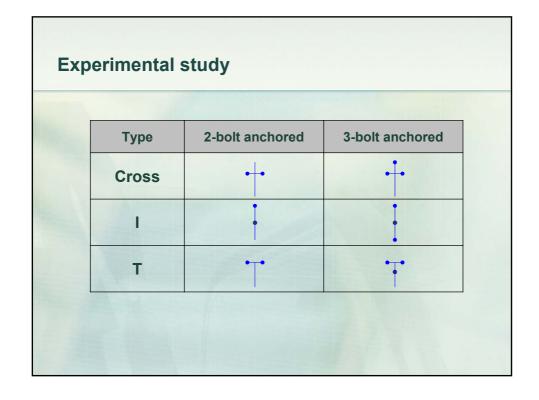


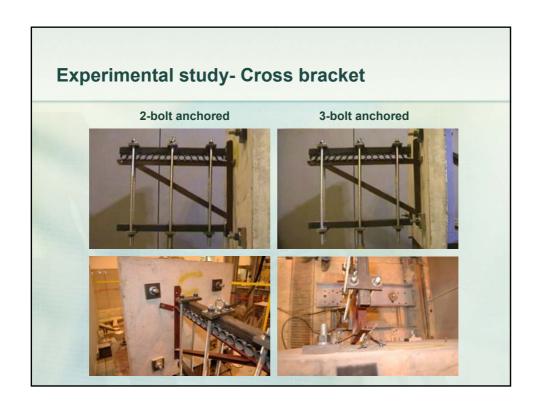


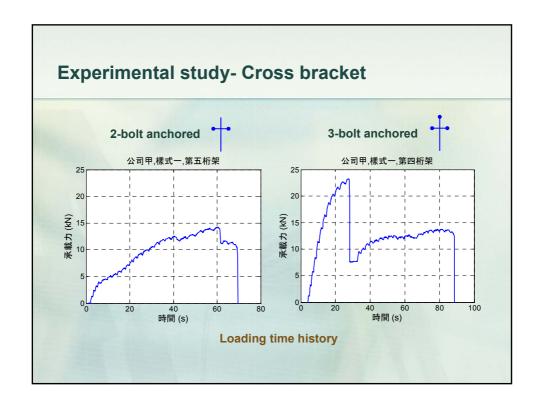


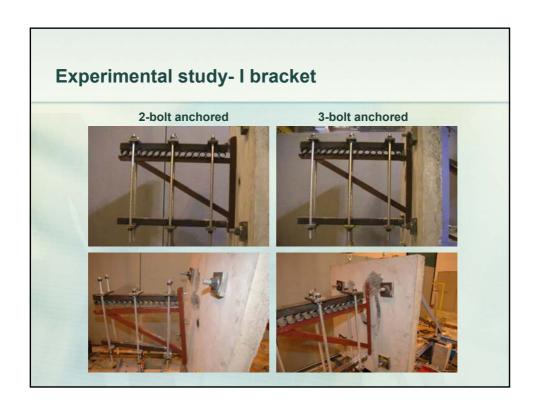


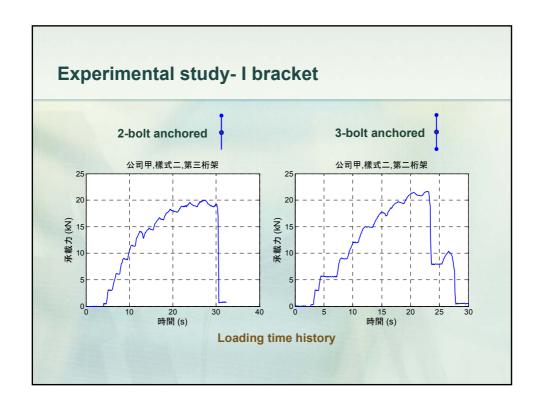


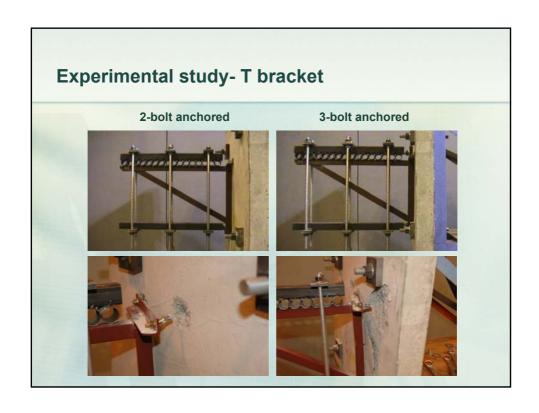


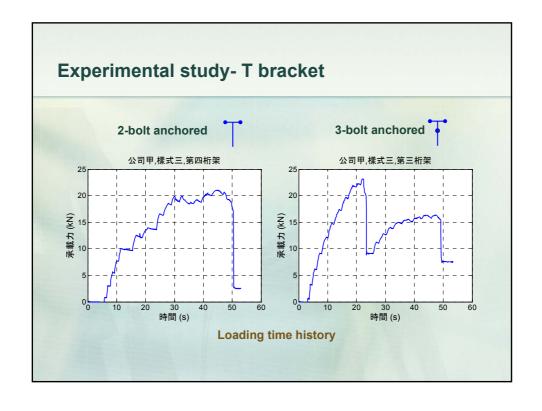












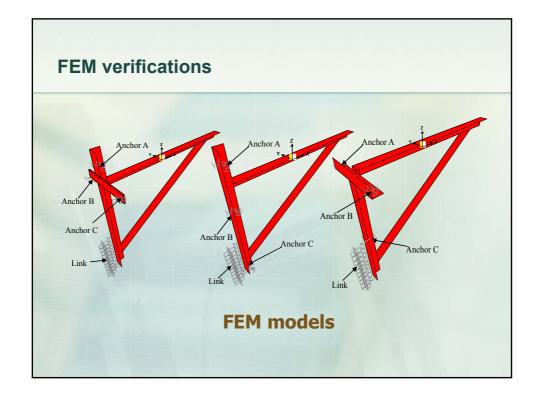
Test results

Load-carrying capacity (in kN)

Type	Anchor	Cross bracket	I bracket	T bracket
Company A/	3-bolt	23	22	23
NLC	2-bolt	14	20	21
Company B/	3-bolt	25	29	33
NLC	2-bolt	15	30	27
Company C/	3-bolt	29	31	30
NLC	2-bolt	18	25	25

Smaller





Comparisons

Туре	Anchor	Cross bracket		I brad	cket	T bracket	
		Test	FEM	Test	FEM	Test	FEM
Company A/ NLC	3-bolt	23	22.2	22	20.3	23	24.5
	2-bolt	14	20.8	20	18.2	21	20.6
Company B/ NLC	3-bolt	25	33.1	29	31.5	33	37.8
	2-bolt	15	33.5	30	28.3	27	32.3
Company	3-bolt	29	33.1	31	31.5	30	37.8
C/ NLC	2-bolt	18	33.5	25	28.3	25	32.3

Conclusions and recommendations

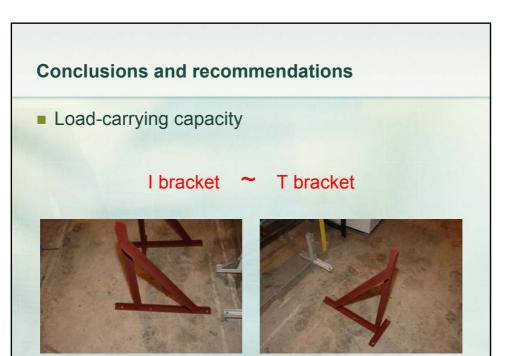
Smaller

 The load-carrying capacity of the Cross bracket is the smallest among the three types





Match



Conclusions and recommendations

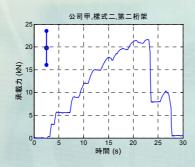
3-bolt anchored is always better than 2-bolt anchored

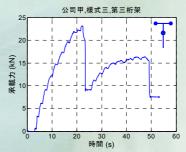
I Good

Туре	2-bolt	3-bolt
Cross	-	•
1		
Т		1

Conclusions and recommendations

- When anchored by 2 bolts, no residual loadcarrying capacity is observed
- When anchored by 3 bolts, T bracket exhibits significant residual load-carrying capacity





Conclusions and recommendations

- T bracket
 - Same level of load-carrying capacity as that of I bracket
 - Provides more significant residual capacity if anchored by 3 bolts



